What is a Retaining wall?

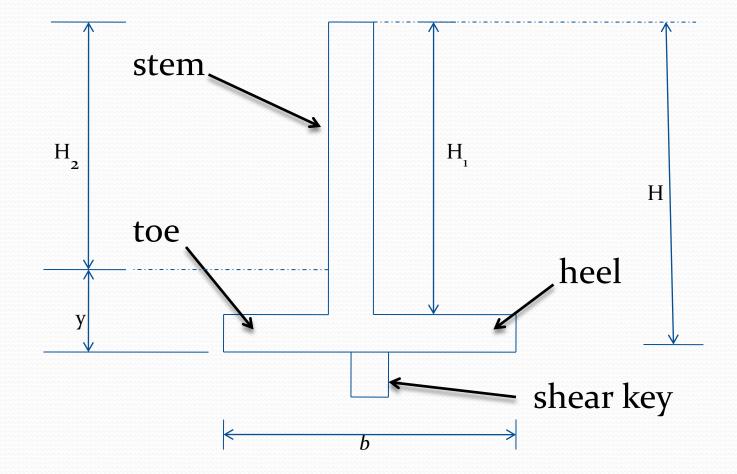
- Retaining wall is a structure used for maintaining the ground surfaces at different elevations on either side of it.
- Retaining walls provide lateral support to vertical slopes of soil. They *retain* soil which would otherwise collapse into a more natural shape. The retained soil is referred to as *backfill*.



Types of retaining walls:

- Gravity Retaining Walls
- Semi-Gravity Retaining Walls
- Cantilever Retaining Walls
- Counter fort Retaining Walls

Cantilever Retaining Wall:



Forces acting on the retaining wall:

- <u>Lateral forces</u>: Earth pressure due to backfill and surcharge.
- Vertical forces:
 - Acting downwards:

Self weight of the retaining wall ; Weight of soil above heel slab.

Acting upwards:

Force due to soil pressure underneath the base slab.

Earth pressures:

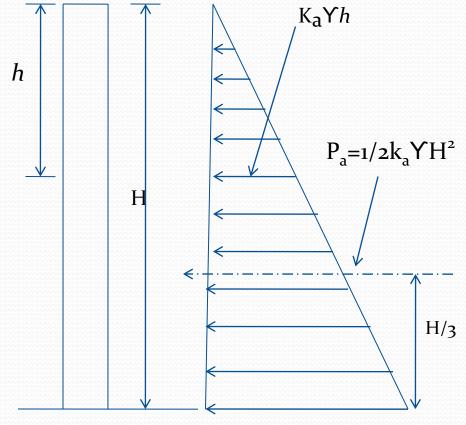
 (a) <u>On stem</u>: Earth pressure on the stem from backfill (active earth pressure) varies linearly. According to Rankine's theory at depth 'h' below the top of wall is given by

 $p_a = k_a \Upsilon h$ where k_a = Coefficient of active earth pressure

$$k_a = 1 - \sin \Phi$$

1+sin Φ

Φ= Angle of internal friction of soilY= Unit weight of back fill

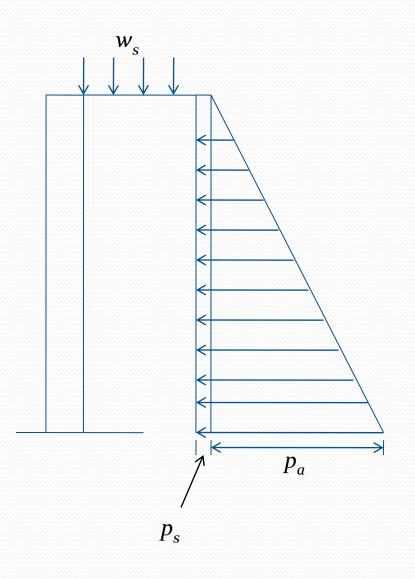




Incase of backfill with surcharge;

- The surcharge on backfill may be due to traffic load on top of back fill or due to a structure near it.
- If w_s is the surcharge pressure on horizontally finished back fill, then uniform effect of surcharge on stem is given by;

$$p_{s} = k_{a} w_{s}$$



If backfill is sloping;

 For sloping black fill, the pressure on stem is parallel to top surface and is given by;

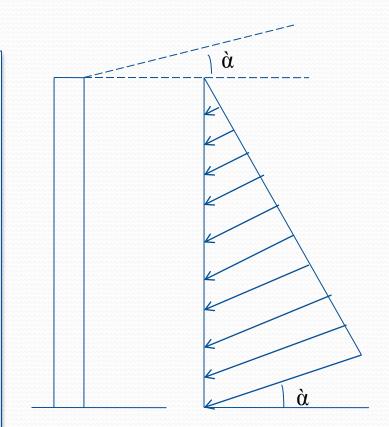
 $p_a = k'_a Y h$

$$k'_{a} = \cos \dot{\alpha} \quad \frac{\cos \dot{\alpha} - \sqrt{\cos^{2} \dot{\alpha} - \cos^{2} \Phi}}{\cos \dot{\alpha} + \sqrt{\cos^{2} \dot{\alpha} - \cos^{2} \Phi}}$$

where,

'α' is angle of slope of backfill with horizontal.(*also reffered as surcharge angle*)

k' is coeff. Of active earth pressure for such case.



Stability Conditions:

- A retaining wall must be stable as a whole, and it must have sufficient strength to resist the forces acting on it.
- In order that the wall may be stable, the following conditions should be satisfied:
- *i.* The wall must be strong enough to resist the bending moment and shear force.
- *ii.* The wall should not overturn.
- *iii.* Maximum pressure at base should not exceed the SBC of soil.
- *iv.* The wall should not slide due to lateral pressure.

Design of RCC Cantilever Retaining walls:

 The depth of foundation depends on the properties of soil. The minimum depth of foundation is calculated from *Rankine*'s formula as

 $y_{min} = q_0 k_a^2 / \Upsilon$

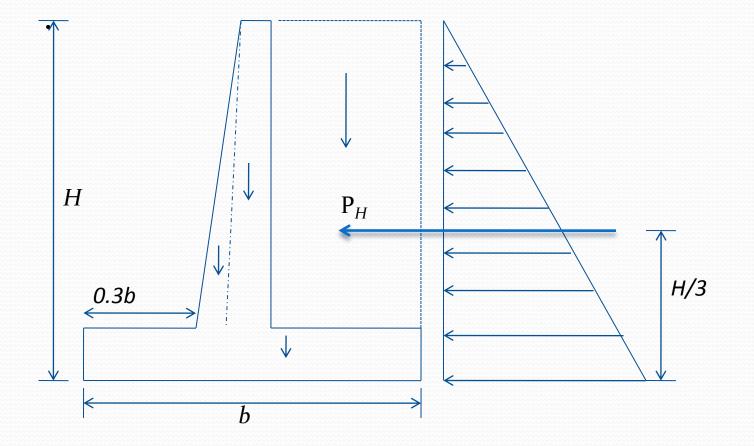
' q_o ' is SBC of soil.

'Y' is the unit weight of soil on which footing is resting. ' k_a ' is the coefficient of active earth pressure.

Preliminary Dimensions:

- The tentative proportions of the cantilever retaining wall may be obtained based on experience and optimization studies.
- Set the preliminary dimensions of retaining wall
- Base width, *b* = 0.48H *to* 0.56H
- Toe projection = 0.3 b
- Thickness of base slab = Thickness of stem = H/12
- Top width of stem = 150 mm to 300 mm

Diagrammatic Representation:



Check for Overturning :

- The lateral loads (earth pressure) causes overturning moment (M_o) about the toe.
- The weight of backfill, surcharge, self weight of retaining wall cause stabilizing moment (M_s) about the toe.
- The factor of safety against overturning is given by ;

 $(Fos)_o = Ms/Mo$

- The factor of safety should not be less than 1.4.
- As per IS 456-2000 recommendations, only 0.9 times the characteristic dead load shall be considered
 (Fos)_o = 0.9Ms/Mo

Check for Sliding :

- The lateral earth pressure on stem tries to slide the retaining wall away from back fill.
- This lateral force is resisted by frictional force between base slab and the soil below it.
- Maximum frictional force is given by

 $F = \mu \Sigma W$

where, ΣW is the total downward load.

 If P_H is the total horizontal pressure, then factor of safety against sliding is given by

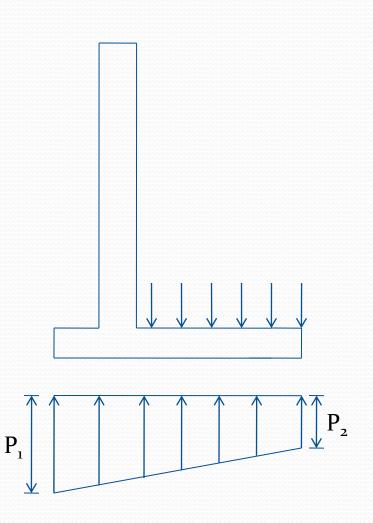
 $(Fos)_s = \mu \Sigma W/P_H$

 As per IS 456-2000 recommendations, the factor of safety should not less than 1.4 and only 0.9 times characteristic dead load is to be considered

 $(Fos)_s = 0.9 \mu \Sigma W/P_H$

Check for Soil Pressure:

- The soil pressure varies linearly with more pressure on toe and less pressure on the end of heel.
- P1 (max.) < SBC of soil.
- P2(*min.*) > 0.



Design of stem:

- Calculate the max. factored BM on stem due to lateral earth pressure. This calculated BM < M_u (*lim.*).
- If cal.BM > M_u (*lim.*) ; increase the thickness of base of stem and redesign.
- Accordingly, calculate the area of steel required;

 $M_u = .87f_y A_{st} d (1 - f_y A_{st}/f_{ck} bd)$

• Provide bars of app. diameter (Φ) and calculate spacing as:

$$S = \frac{\prod \Phi^2/4}{A_{st}} * 1000$$

- Spacing should be min. of the following:
 (1) 0.75d (2) 300mm (3) Calculated Spacing
- Provide *distribution steel.*
- Check for development length and shear.

Design of toe slab:

- Calculate the ultimate BM for *1 metre* width of toe slab.
- For calculation of BM, The weight of soil above toe slab is neglected. The two forces considered are:

 (1) Upward soil pressure;
 (2) Downward weight of toe slab.
- Provide reinforcement accordingly.
- Provide *distribution steel*.
- Check for *development length* and *shear*.

Design of heel slab:

- Calculate the ultimate BM for *1 metre* width of heel slab.
- For calculation of BM,
 - The three forces considered are:
 - (1) Upward soil pressure;
 - (2) Downward weight of heel slab;
 - (3) Weight of the soil above heel slab.
- Provide *main steel* and *distribution steel* accordingly.
- Apply check for *development length* and *shear*.

